## PROPOSED MIXED USE DEVELOPMENT

1 VILLAWOOD PL, VILLAWOOD

Assessment of Traffic and Parking Implications

October 2015 (Rev E)

Reference 14195

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FIGURE 1	LOCATION

FIGURE 2 SITE
FIGURE 3 ROAD NETWORK
FIGURE 4 TRAFFIC CONTROLS

#### 1. Introduction

This report has been prepared to accompany revised plans for a Development Application to Fairfield City Council for a proposed mixed use development in Villawood Place at Villawood (Figure 1).

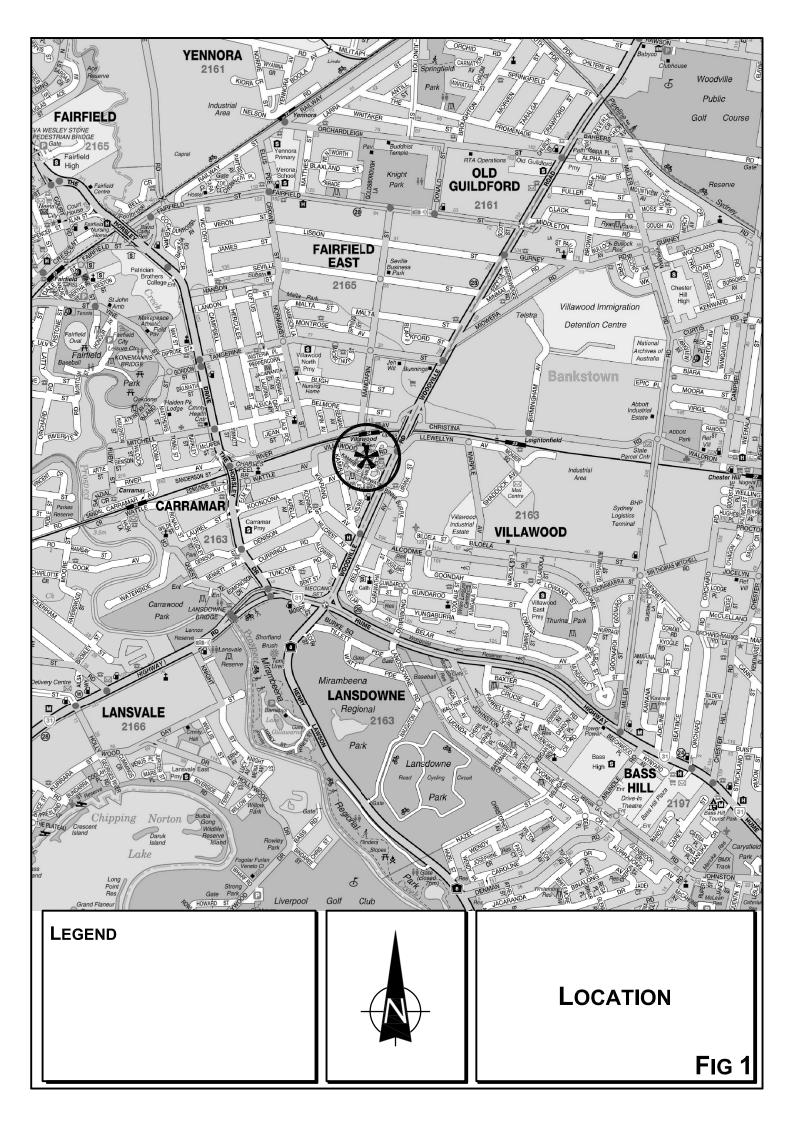
The site is located within the Villawood Town Centre and within close proximity to the railway station and bus services therefore presenting a suitable location for a mixed use development as part of the urban consolidation process.

The proposed development scheme comprises:

- **★** 119 residential apartments
- \* Retail units (including a cafe)
- \* Basement car parking

The purpose of this report is to:

- \* describe the site, its context and the development proposal
- \* describe the existing road network and conditions on that network
- \* assess the adequacy of the proposed parking provision
- assess the potential traffic implications
- \* assess the proposed access and internal circulation and servicing arrangements.



#### 2. Proposed Development Scheme

#### 2.1 SITE, CONTEXT AND EXISTING USE

The development site (Figure 2) is Lot 4 in DP 1013056 which occupies an area of some 2,324 m<sup>2</sup> with frontages to Villawood Place and Kamira Circuit. The site is located within the Villawood Town Centre just to the south of the Villawood Railway Station. The existing building on the site, which is now vacant, formerly comprised a supermarket and specialty shops (total 1,890 m<sup>2</sup>) with 30 on-site car parking.

The surrounding uses comprise a number of industrial properties to the east, a 'KFC' restaurant and Aldi Supermarket to the west. There is a council carpark which adjoins the site to the south west with 76 spaces accessed on Kamira Circuit.

#### 2.2 PROPOSED DEVELOPMENT

It is proposed to demolish the existing building and excavate the site to construct a new 8-storey building with 3 basement levels. The proposed development will comprise:

#### Residential

12 x 1 bed apartments

95 x 2 bed apartments

12 x 3 bed apartments

Total: 119 apartments

#### **Commercial**

8 x retail units (1,137 m<sup>2</sup> GFA)

1 x community centre (184.4 m<sup>2</sup> GFA)

The basement carpark will provide 170 spaces with access on Kamira Circuit.





SITE

FIG 2

#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

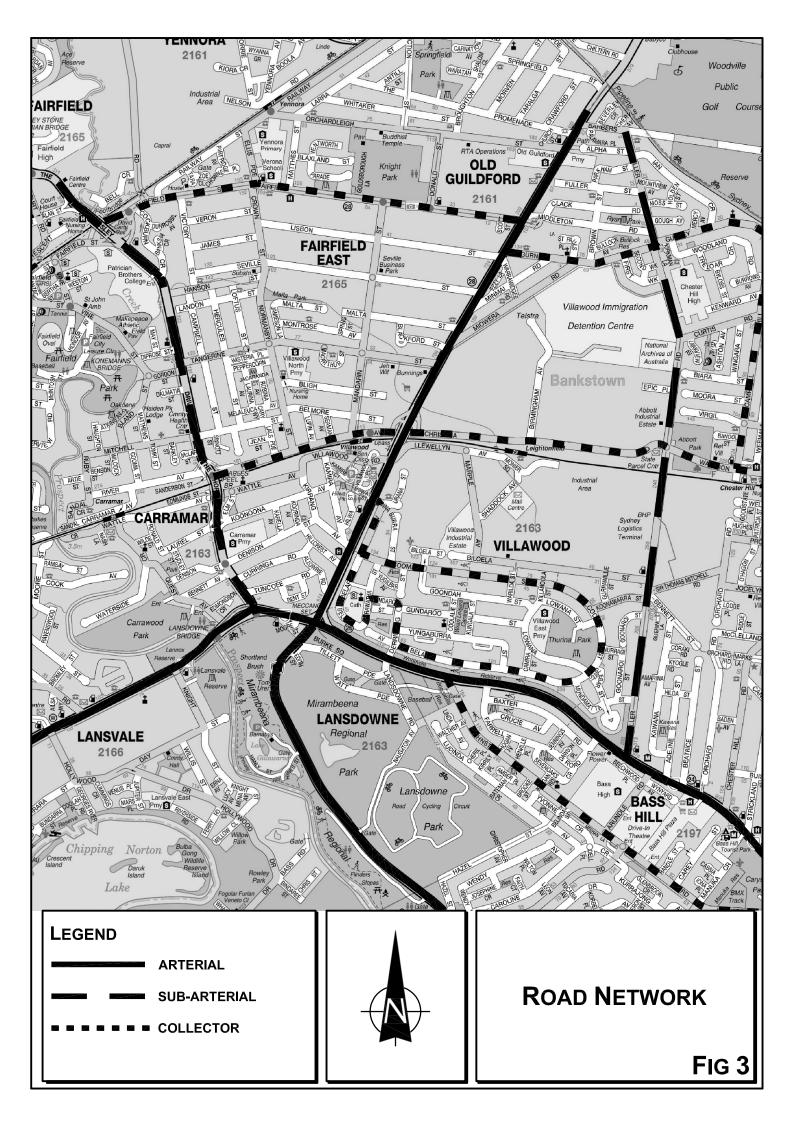
Architectural details are provided on the revised plans prepared by Tony Owen Partners, which accompany the Development Application and are reproduced in part in Appendix A.

#### 3. Road Network and Traffic Conditions

#### 3.1 ROAD NETWORK

The road network serving the site (Figure 3) comprises:

- \* Hume Highway a State Highway and arterial route for both inter and intra state travel
- ★ Woodville Road a State Road and arterial route connecting between Great Western Highway/M4 and Hume Highway
- ★ Henry Lawson Drive a State Road and sub-arterial route connecting between Hume Highway and M5/Milperra Road
- Christina Road part of a Regional Road and collector route connecting between Carramar and Regents Park which interchanges with Woodville Road (via on/off ramps)
- ★ The Horsley Drive a sub-arterial road connecting between Hume Highway
  Cumberland Highway
- \* Llewellyn Avenue, Villawood Road and Wattle Avenue minor collector routes connecting between Villawood and Carramar.
- Villawood Place/Howatt Street- part of a local road system off Woodville Road providing access to the local Villawood shops
- \* Kamira Circuit— a local one way access road connecting between Kamira Avenue and Villawood Road.



#### 3.2 TRAFFIC CONTROLS

The existing traffic controls which have been applied to the road network serving the site (Figure 4) include:

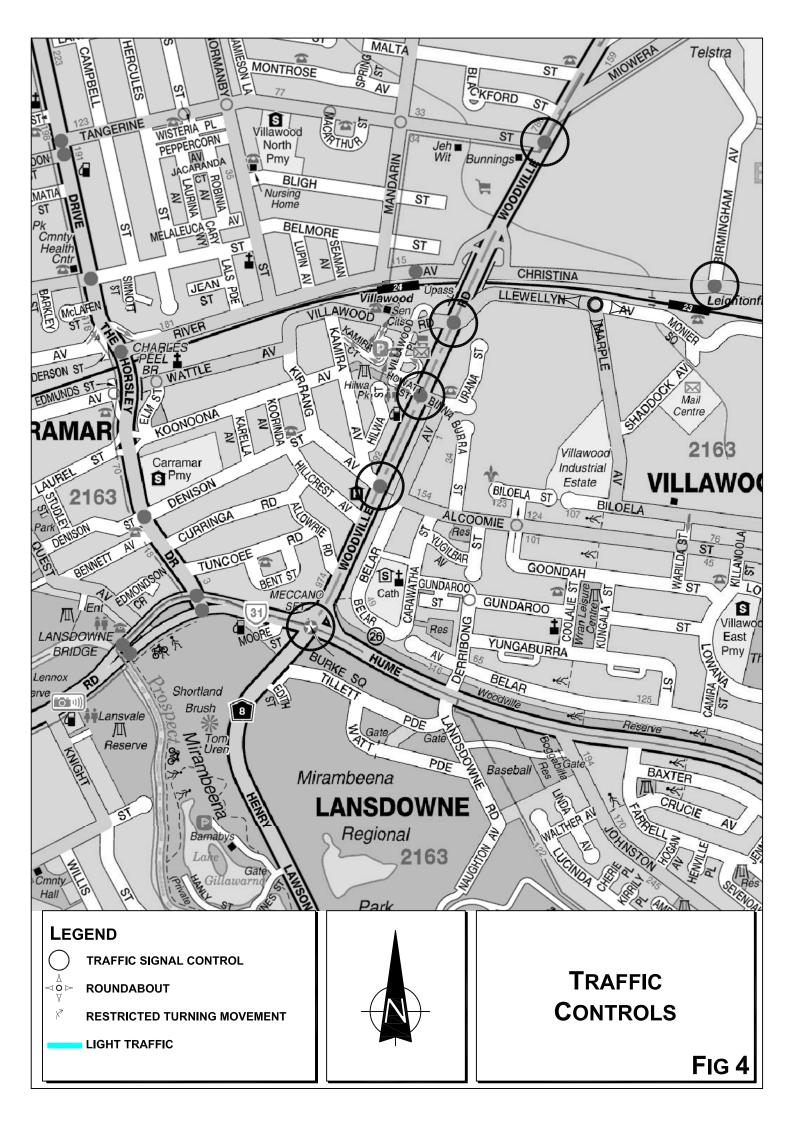
- \* the traffic signal controls at the Woodville Road, Villawood Road, and Llewellyn Ave intersections
- \* the pedestrian mid-block traffic signals on River Avenue just to the north of the station
- \* the other traffic signals along Woodville Road at the Binna Burra Street, Kirang Avenue and Hume Highway intersections
- \* the roundabout at the Llewellyn Avenue and Marple Avenue intersection
- \* the sections of NO STOPPING restrictions along Villawood Road including along the site frontage
- \* the one way restriction in the Kamira Circuit carpark
- \* the marked pedestrian crossings at Villawood Road and Villawood Place along the site frontages.

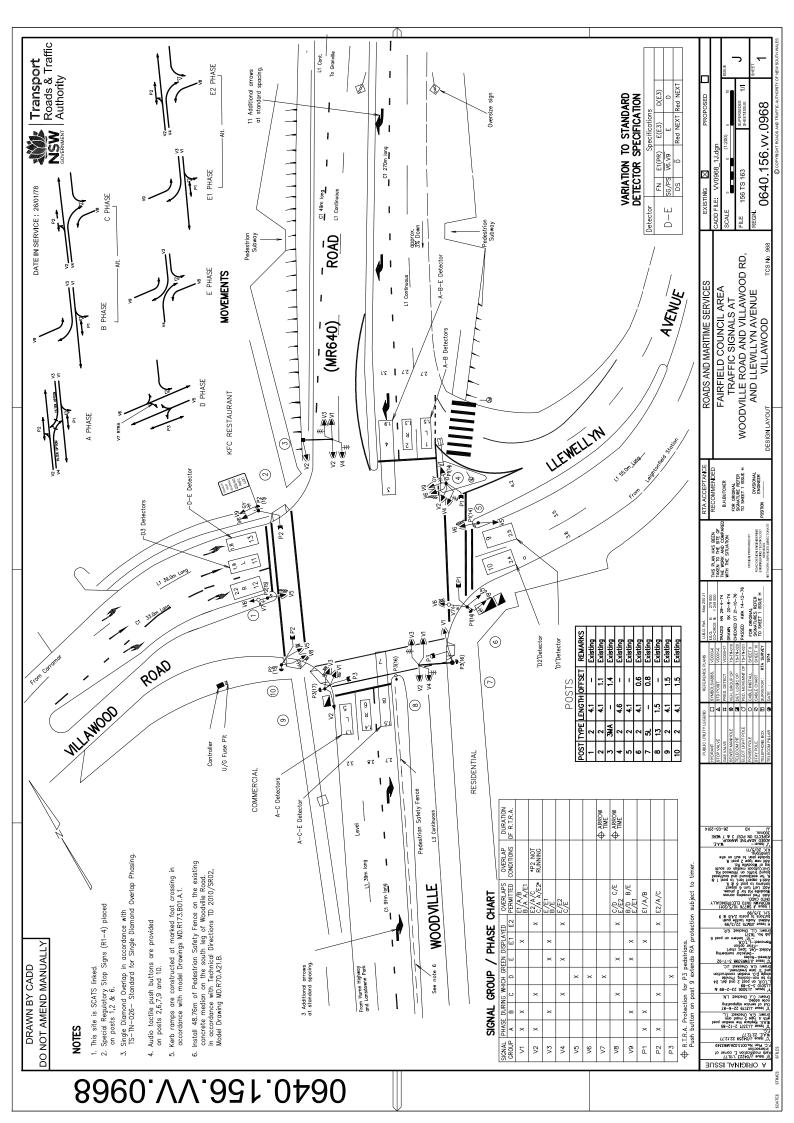
#### 3.3 TRAFFIC CONDITIONS

An indication of traffic conditions on the road system serving the site is provided by data published by the Roads and Maritime Services (RMS). The RMS data<sup>1</sup> is expressed in terms of average annual daily traffic (AADT) and the following flows which are consistent with the function of the roads are summarised in the following:

Woodville Road, south of Christina Road 44,255 Hume Highway east of Woodville Road 48,616

<sup>&</sup>lt;sup>1</sup> Traffic Volume Data for Sydney Region Roads and Maritime Services





#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

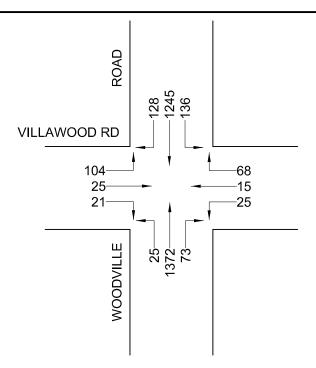
Recent traffic surveys undertaken at the Villawood Road/Woodville Road intersection during the morning and afternoon peak periods (Figure 5) indicate the following:

	Movement	AM	PM
Woodville Road			
North	Left	136	52
	Through	1245	1602
	Right	128	215
Llewellyn Road			
East	Left	25	125
	Through	15	105
	Right	68	130
Woodville Road			
South	Left	25	61
	Through	1372	1068
	Right	73	35
Villawood Road			
West	Left	104	126
	Through	25	25
	Right	21	102

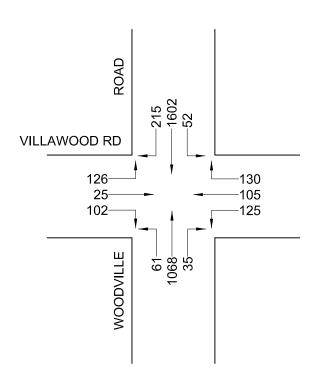
Assessment of operation performance of the above intersection using SIDRA indicates satisfactory peak hour levels of service as follows:

	AM			PM	
DS	LOS	AVD	DS	LOS	AVD
0.649	В	17.7s	0.775	В	25.0s

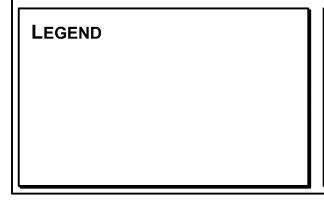
It is apparent that the current intersection is operating within capacity and while there are some vehicle queues along Woodville Road during the peak hours this is entirely consistent with the arterial function of Road.



## AM PEAK



## PM PEAK





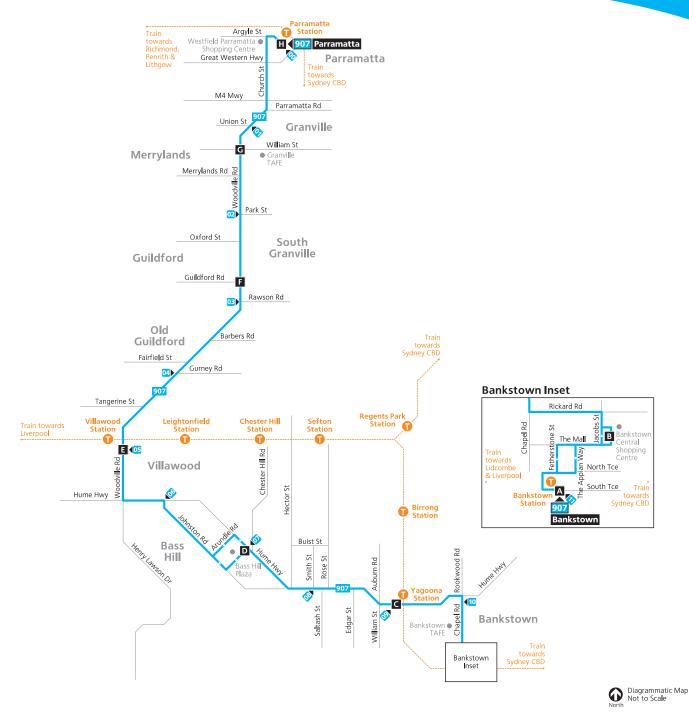
# PEAK TRAFFIC VOLUMES

FIG 5

#### 3.4 TRANSPORT SERVICES

Villawood Railway Station (100 metres to the north along Villawood Place) provides high frequency train services while bus services (route 907) along Woodville Road connecting Bankstown and Parramatta. As a result the site is considered to be well serviced by public transport.









A Timing point

O4 Section point





#### 4. PARKING

Fairfield City Council's Development Control Plan details the following parking criteria:

#### **Residential (Shop Top Housing)**

1-2 beds 1 space 3 beds or more 1 space

Visitors 1 space per 4 dwellings

Commercial/Retail

Shops 1 space per 40 m<sup>2</sup>

It is understood from Council's correspondence dated 24 February 2015 that parking requirements associated with commercial development would be provided at a rate of 1 space per 63 m<sup>2</sup> up to a limit of 1,890 m<sup>2</sup>. Application of this along with DCP criteria to the proposed development scheme would indicate a requirement of:

119 x apartments119 spacesVisitors30 spaces

Commercial (1,137 m<sup>2</sup>) 18 spaces Community (185 m<sup>2</sup>) 3 spaces

Total 170 spaces

Having regard to the above it is proposed to provide 170 spaces in the basement according to the specified allocation. It is apparent that the proposed provision of parking spaces will be compliant with the Council's criteria, and will be appropriate for the locality of the site.

#### 5. TRAFFIC

An indication of the potential traffic generation of the proposed development is provided by the RMS Development Guidelines (technical Direction TDT 2013/4b) indicates a generation during the on-street peak traffic periods of:

119 apartments @

AM 0.19 (23 vtph)

PM 0.15 (18 vtph)

The projected distribution of the residential component is:

	AM .	F	PM
IN	OUT	IN	OUT
6	17	12	6

The traffic generation criteria contained in RMS' Development Guidelines for retail uses is for large regional shopping centres and is not applicable for small retail space in a mixed use development which is largely convenience and ancillary. It is more appropriate to apply a 'parking space' turnover and surveys of comparable developments (ie small supermarkets within town centre locations) has established parking space turnover rates as follows:

AM	PM
20%	50%

Council's parking requirement for the non-residential component of 21 spaces would reflect a turnover i.e. a 'visitation' equivalent to a peak of some 5 and 11 vtph distributed in the following:

#### TRANSPORT AND TRAFFIC PLANNING ASSOCIATES

	Į.	MА	F	PM
	IN	OUT	IN	OUT
Retail	5	5	11	11

As a result the total projected movements (without discount in relation to the generation of former uses) are as follows:

	A	M	F	PM
	IN	OUT	IN	OUT
Retail	5	5	11	11
Residential	6	17	12	6
Total	11	22	23	17

Whilst in reality not all the projected traffic would access the site via the Villawood Road/Woodville Road intersection, a conservative SIDRA assessment assuming 100% access movements via the key intersection was undertaken. The assessment indicates minor delays on the signal operation with no perceptible traffic or safety implications on the surrounding traffic network as follows:

		AM		PM				
	DS	LOS	AVD	DS	LOS	AVD		
Existing	0.649	В	17.7s	0.775	В	25.0s		
Development	0.650	В	17.9s	0.785	В	26.1s		

Details of the SIDRA output are provided in Appendix B while a guide to interpret SIDRA results is provided overleaf.

#### 6. Access, Internal Circulation and Servicing

#### **Access**

Access to the basement carpark will involve a 5.5 metre wide combined ingress/egress driveway. The design of the access driveway will accord with the requirements of AS2890.1 and it will be located where good sight distance is available particularly for egressing drivers.

#### INTERNAL CIRCULATION

The proposed carpark is to be provided on 3 basement levels and the internal circulation arrangements both within and between each level, including ramp grades, aisle widths and parking bay dimensions will accord with the requirements of AS 2890.1 and 6. Details of concurrent car passing where ramps or circulation roads intersect with parking aisles are provided in Appendix C.

#### **SERVICING**

Refuse removal will be undertaken by an 8.8m MRV at the designated garbage storage area during off peak hours. The service vehicle will undertake 1 reverse movement to enter the loading dock and exit forwards. Occasional servicing activities including retail deliveries, small furniture and tradespersons vehicles movements including infrequent need for larger service vehicles will also be satisfied by use of the loading area. Small service vehicles (eg. service personnel) will also be able to use the visitor spaces. Although the heavy vehicles will only be reversing onto the docking area outside of core business hours, it is advised that such maneuver be supervised by a 'spotter' to ensure safety of other road users at all times.

Details of satisfactory internal circulation are provided on the turning path assessment in Appendix C.

#### 8. ISSUES

The traffic and parking issues raised by Council are responded to in the following:

#### \* Small car spaces

There are no small car spaces in the revised plans.

#### Wider parking bays

All parking bays adjacent to walls etc leave +300mm width in the revised plans.

#### Head Room

The head room clearances in the revised plans comply with AS2890.1

#### Vehicle Passing

In section 2.5.2 of AS2890.1 the requirements for concurrent vehicle passing only relate to "Intersections between circulation roadways and ramps with parking aisles" (ie not for parking aisle with parking aisle)

The ability for 99<sup>th</sup> and 85<sup>th</sup> percentile cars to pass at the specified locations within the basement levels is shown on the relevant turning path diagram in Appendix B.

#### Corner Splays

A 3m x 3m splay has been provided at the Kamira Court intersection and a 2m x 2.5m splay at the car park egress

#### 7. SERVICE DOCK MANAGEMENT PLAN

- Access for the service dock will be located on Kamira Circuit and the roller shutter access will remain open between 6.30am and 11.00pm (7 days).
- The service dock will provide for:
  - retail deliveries
  - refuse removal (retail and residential apartment)
  - deliveries and removals for residential apartments
  - emergency and maintenance vehicles
- Service personnel and other small vehicles (couriers and small van deliveries)
   will also be able to use the retail/visitor parking spaces.
- The Building Manager will supervise the dock activity. Should a vehicle arrive
  that cannot be accommodated in the dock due to the presence of other vehicles
  the Building Manager will direct the driver to depart and contact him by mobile
  phone to confirm the ability to return.
- The Building Manager will ensure that drivers do not "overstay" in the loading dock.
- The loading dock will accommodate access and manoeuvring for up to an 8.8m
   truck with a height clearance of 4.5m.
- Residents requiring deliveries or removals will organize times for these activities with the Building Manager.
- Refuse removal times will be directed by the Building Manager for non-peak times.
- No "out of hours" retail delivery will be permitted.

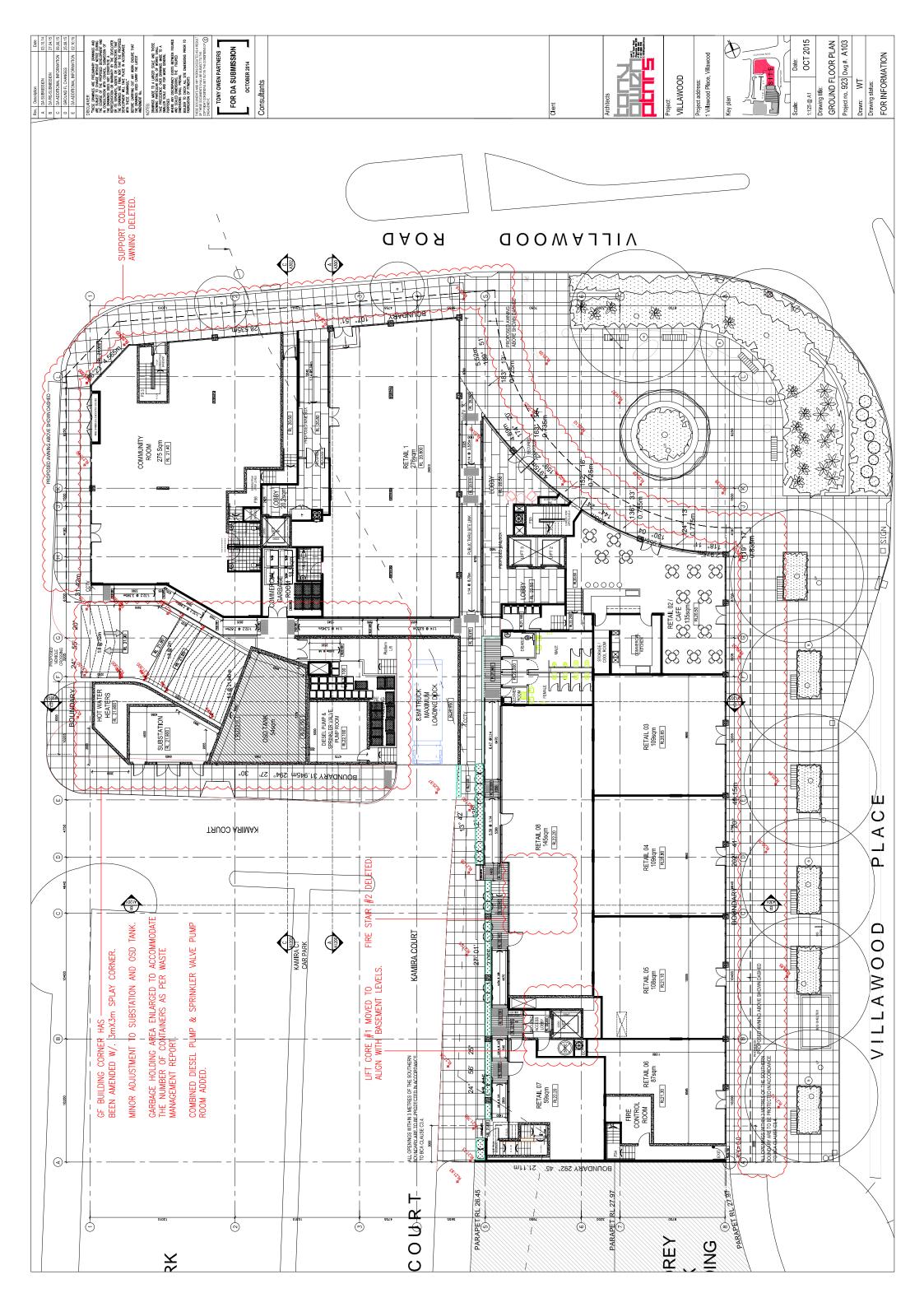
#### 9. CONCLUSION

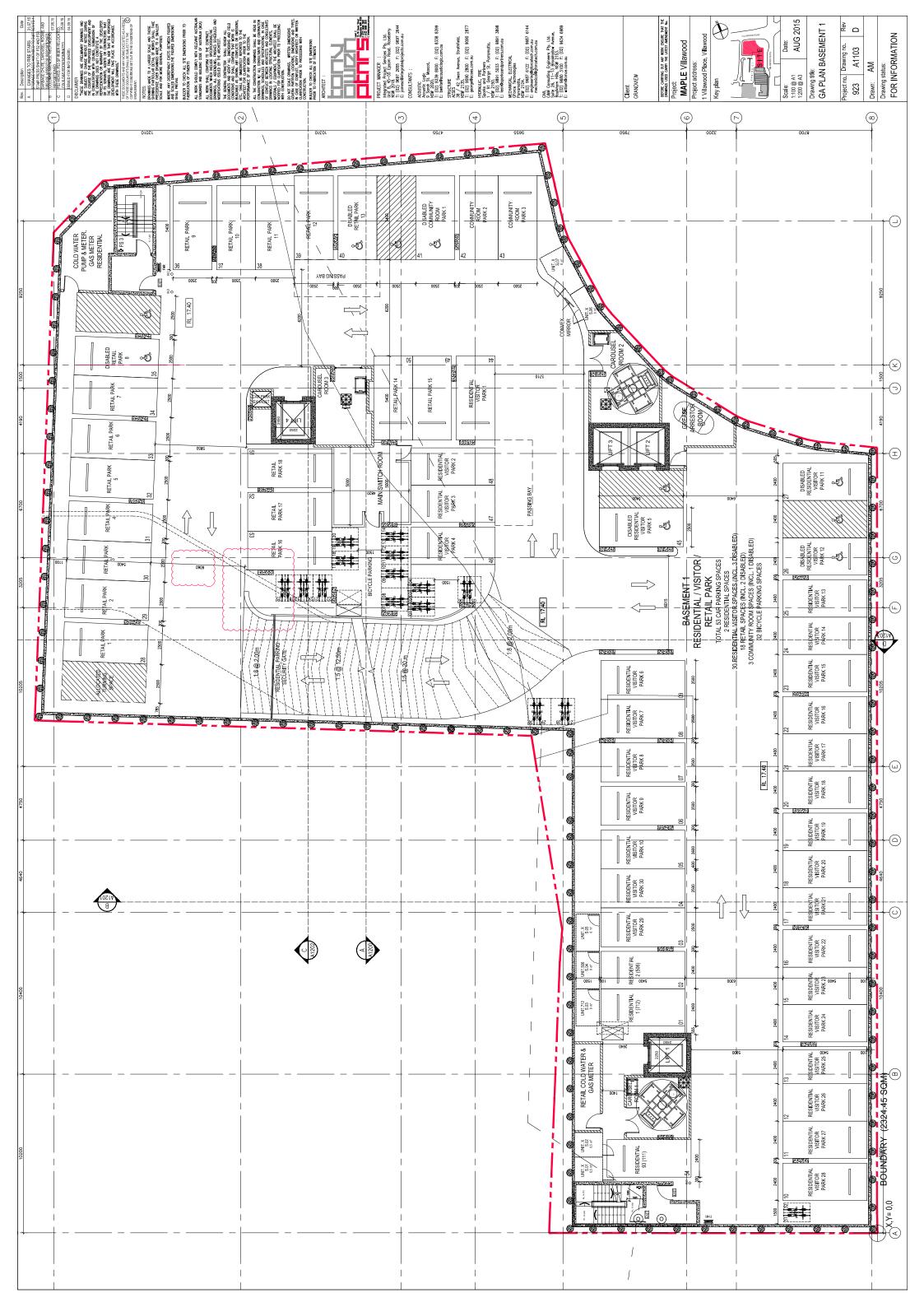
The proposed mixed use development on the site formerly occupied by a supermarket and specialty shops in Villawood Place at Villawood represents an appropriate redevelopment outcome which will be consistent and compatible with the planning for development in the area. The assessment of the potential traffic, transport and parking implications of the development scheme has concluded that:

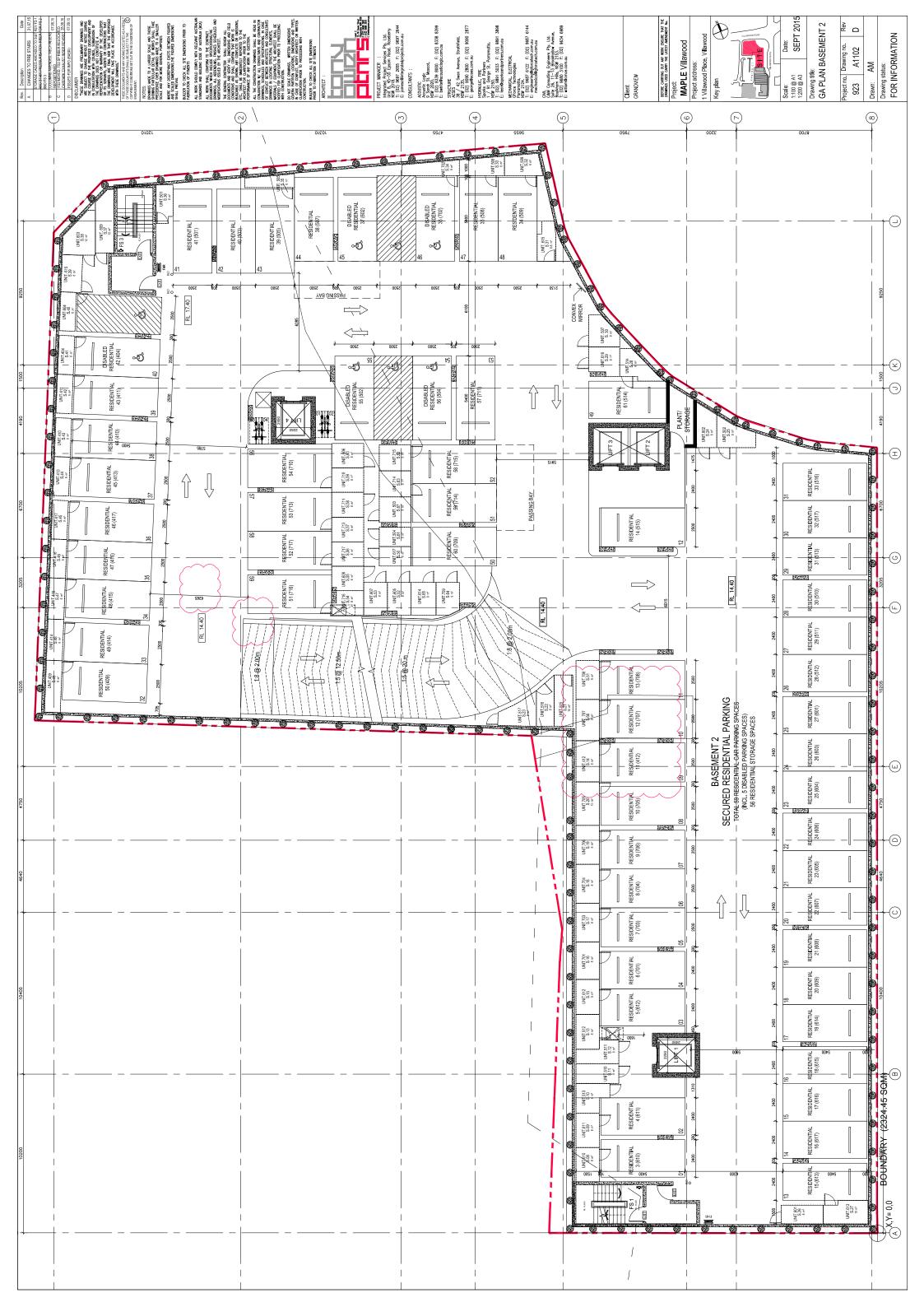
- \* the proposed vehicle access, internal circulation and servicing arrangements will be suitable and appropriate
- there will not be any unsatisfactory traffic implications
- \* the proposed parking provision will be adequate and appropriate to the circumstances.

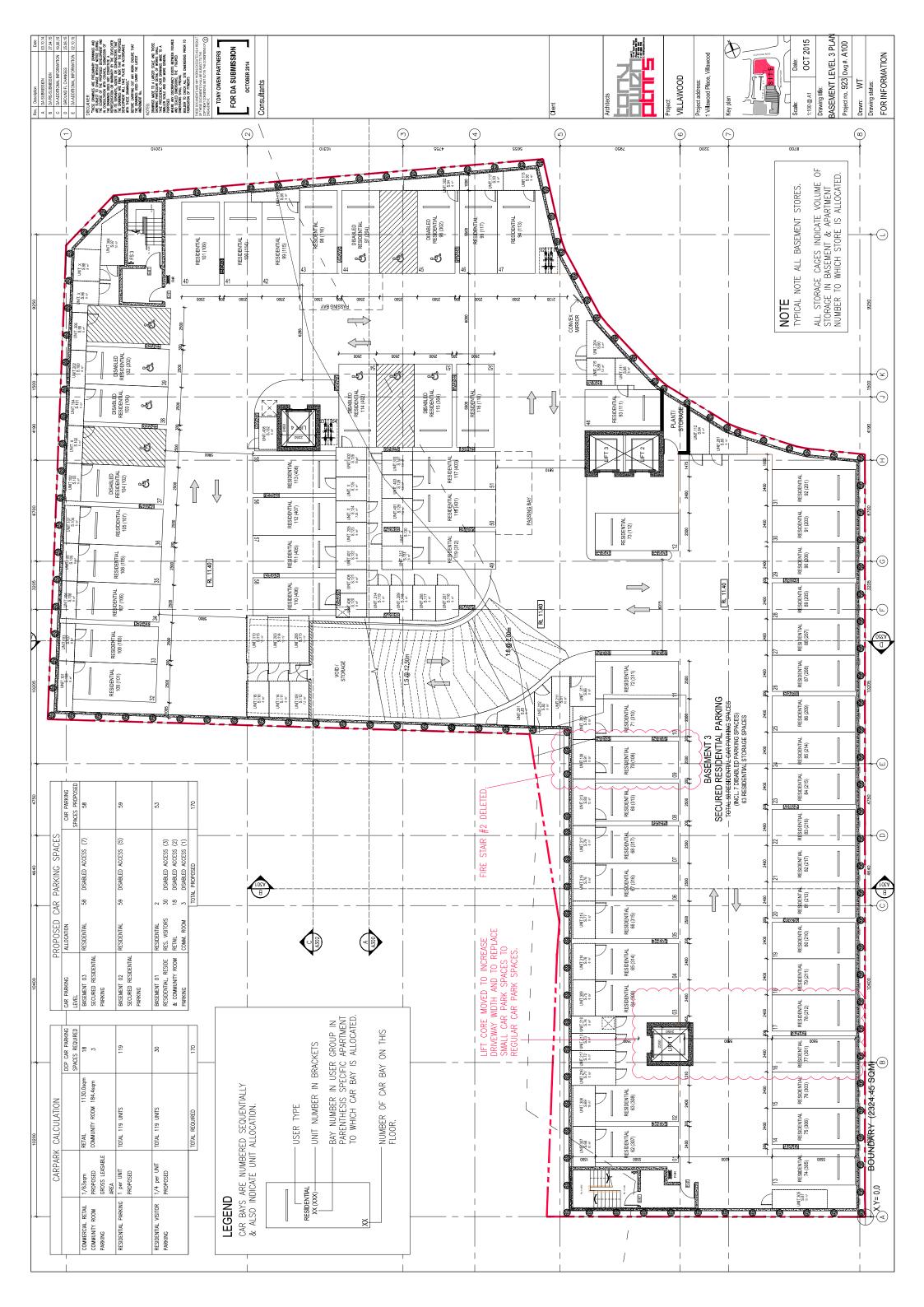
## **APPENDIX A**

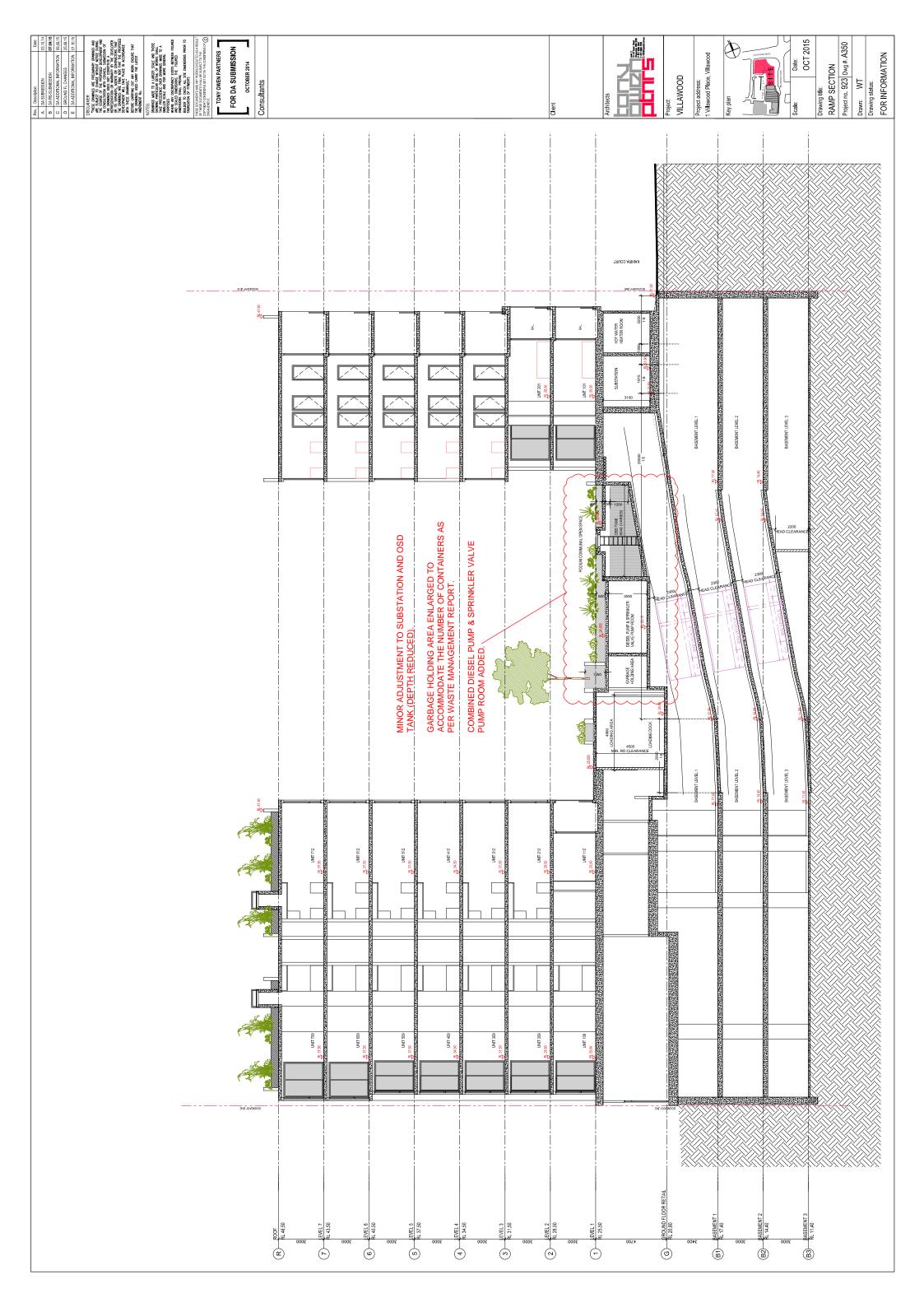
**ARCHITECTURAL PLANS** 











## **APPENDIX B**

**SIDRA OUTPUT** 

Site: VILLAWOOD AND **WOODVILLE AM** 

New Site

Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Moven	nent Pe	rformance -	Vehicles								
	_	Demand	1.07	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Caudh. V	WOODV	veh/h	%	v/c	sec		veh	m		per veh	km/h
			0.0	0.504	45.0	1.00.0	47.0	100.0	0.47	4.00	40.5
1	L	25	2.0	0.501	15.8	LOS B	17.3	122.9	0.47	1.00	43.5
2	Т	1372	2.0	0.501	7.5	LOS A	17.3	123.1	0.47	0.43	47.9
3	R	73	2.0	0.226	16.0	LOS B	1.6	11.7	0.45	0.75	41.6
Approa	ch	1470	2.0	0.501	8.1	LOS A	17.3	123.1	0.47	0.45	47.5
East: Ll	LWELLY	N									
4	L	25	2.0	0.454	60.6	LOS E	6.0	42.5	0.96	0.79	22.6
5	Т	15	2.0	0.454	52.3	LOS D	6.0	42.5	0.96	0.77	22.8
6	R	68	2.0	0.454	60.4	LOS E	6.0	42.5	0.96	0.79	22.7
Approa	ch	108	2.0	0.454	59.3	LOS E	6.0	42.5	0.96	0.79	22.7
North: V	NOODVI	LLE									
7	L	136	2.0	0.285	8.3	LOS A	0.5	3.7	0.20	0.64	48.7
8	T	1245	2.0	0.446	7.1	LOS A	14.5	103.1	0.44	0.40	48.5
9	R	128	2.0	0.462	20.4	LOS B	4.3	30.6	0.63	0.79	38.5
Approac	ch	1509	2.0	0.462	8.3	LOS A	14.5	103.1	0.43	0.45	47.5
West: V	/ILLAWO	OD									
10	L	104	2.0	0.325	55.5	LOS D	5.4	38.5	0.92	0.78	23.7
11	Т	25	2.0	0.173	58.2	LOS E	1.4	10.1	0.97	0.69	22.3
12	R	21	2.0	0.158	66.5	LOS E	1.2	8.5	0.97	0.71	21.2
Approa	ch	150	2.0	0.325	57.5	LOS E	5.4	38.5	0.93	0.76	23.1
All Vehi	cles	3237	2.0	0.501	12.2	LOSA	17.3	123.1	0.49	0.48	43.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movem	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate					
		ped/h	sec		ped	m		per ped					
P3	Across E approach	53	6.0	LOS A	0.1	0.1	0.32	0.32					
P7	Across W approach	53	7.7	LOS A	0.1	0.1	0.36	0.36					
All Pedestrians		106	6.9	LOS A			0.34	0.34					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SIDRA INTERSECTION

Site: VILLAWOOD AND **WOODVILLE PM** 

New Site

Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Movement Performance - Vehicles											
		Demand	107	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	WOODVI	veh/h	%	v/c	sec		veh	m		per veh	km/h
			2.0	0.505	20.5	LOCD	20.7	447.0	0.00	0.04	20.0
1	L	61	2.0	0.535	26.5	LOS B	20.7	147.3	0.69	0.94	36.2
2	T	1068	2.0	0.535	18.2	LOS B	20.8	148.1	0.69	0.62	38.1
3	R	35	2.0	0.173	42.0	LOS C	1.6	11.5	0.81	0.77	27.8
Approa	ch	1164	2.0	0.535	19.4	LOS B	20.8	148.1	0.69	0.64	37.6
East: LI	LWELLYN	I									
4	L	125	2.0	0.757	52.6	LOS D	20.0	142.2	0.97	0.89	24.8
5	Т	105	2.0	0.757	44.4	LOS D	20.0	142.2	0.97	0.87	25.0
6	R	130	2.0	0.757	52.5	LOS D	20.0	142.2	0.97	0.89	24.9
Approa	ch	360	2.0	0.757	50.2	LOS D	20.0	142.2	0.97	0.88	24.9
North: \	WOODVII	LE.									
7	L	52	2.0	0.109	8.0	LOSA	0.2	1.3	0.12	0.62	49.1
8	Т	1602	2.0	0.757	22.1	LOS B	35.7	253.8	0.84	0.77	35.5
9	R	215	2.0	0.737	46.3	LOS D	12.8	91.3	1.00	0.94	26.4
Approa	ch	1869	2.0	0.757	24.5	LOS B	35.7	253.8	0.83	0.78	34.4
West: V	/ILLAWO	OD									
10	L	126	2.0	0.197	37.3	LOS C	5.1	36.3	0.74	0.78	29.6
11	Т	25	2.0	0.061	37.3	LOS C	1.1	7.9	0.80	0.58	28.5
12	R	102	2.0	0.530	58.8	LOS E	5.6	39.9	0.95	0.80	22.9
Approa	ch	253	2.0	0.530	45.9	LOS D	5.6	39.9	0.83	0.76	26.4
All Vehi	icles	3646	2.0	0.757	26.9	LOS B	35.7	253.8	0.80	0.74	33.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians									
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective	
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate	
		ped/h	sec		ped	m		per ped	
P3	Across E approach	53	14.5	LOS B	0.1	0.1	0.49	0.49	
P7	Across W approach	53	17.1	LOS B	0.1	0.1	0.53	0.53	
All Pedestrians		106	15.8	LOS B			0.51	0.51	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: VILLAWOOD AND **WOODVILLE AM DEV** 

New Site

Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Moven	nent Pe	rformance -	Vehicles								
	_	Demand	1.07	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Caudh. V	MOODY	veh/h	%	v/c	sec		veh	m		per veh	km/h
	WOODV		0.0	0.500	45.0	1.00.0	47.0	100.0	0.47	4.00	40.5
1	L	28	2.0	0.502	15.8	LOS B	17.3	123.3	0.47	1.00	43.5
2	Т	1372	2.0	0.502	7.5	LOS A	17.4	123.6	0.47	0.43	47.9
3	R	73	2.0	0.226	16.0	LOS B	1.6	11.7	0.45	0.75	41.6
Approa	ch	1473	2.0	0.502	8.1	LOS A	17.4	123.6	0.47	0.46	47.4
East: Ll	LWELLY	N									
4	L	25	2.0	0.455	60.6	LOS E	6.0	42.5	0.96	0.79	22.6
5	Т	15	2.0	0.455	52.4	LOS D	6.0	42.5	0.96	0.77	22.8
6	R	68	2.0	0.455	60.4	LOS E	6.0	42.5	0.96	0.79	22.7
Approa	ch	108	2.0	0.455	59.4	LOS E	6.0	42.5	0.96	0.79	22.7
North: V	NOODVI	LLE									
7	L	136	2.0	0.285	8.3	LOS A	0.5	3.7	0.20	0.64	48.7
8	T	1245	2.0	0.446	7.1	LOSA	14.5	103.1	0.44	0.40	48.5
9	R	136	2.0	0.492	20.9	LOS B	4.8	34.1	0.66	0.80	38.1
Approac	ch	1517	2.0	0.492	8.4	LOS A	14.5	103.1	0.44	0.46	47.4
West: V	/ILLAWO	OD									
10	L	119	2.0	0.371	56.0	LOS D	6.2	44.5	0.93	0.79	23.6
11	Т	25	2.0	0.173	58.2	LOS E	1.4	10.1	0.97	0.69	22.3
12	R	28	2.0	0.211	66.9	LOS E	1.6	11.5	0.97	0.72	21.1
Approa	ch	172	2.0	0.371	58.1	LOS E	6.2	44.5	0.94	0.77	23.0
All Vehi	cles	3270	2.0	0.502	12.6	LOSA	17.4	123.6	0.50	0.48	43.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians									
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective	
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate	
		ped/h	sec		ped	m		per ped	
P3	Across E approach	53	6.0	LOS A	0.1	0.1	0.32	0.32	
P7	Across W approach	53	7.7	LOS A	0.1	0.1	0.36	0.36	
All Pedestrians		106	6.9	LOS A			0.34	0.34	

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Moven	nent Pe	rformance -	Vehicles								
	_	Demand	1157	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Courthy	WOODV	veh/h	%	v/c	sec		veh	m		per veh	km/h
	, AGOODA		0.0	0.545	07.0	1.00.0	04.0	450.0	0.70	0.00	05.0
1	L -	65	2.0	0.545	27.2	LOS B	21.2	150.8	0.70	0.93	35.8
2	Т	1068	2.0	0.545	19.0	LOS B	21.3	151.7	0.70	0.63	37.6
3	R	35	2.0	0.162	42.7	LOS D	1.6	11.6	0.81	0.77	27.5
Approac	ch	1168	2.0	0.545	20.1	LOS B	21.3	151.7	0.70	0.65	37.1
East: Ll	LWELLY	٧									
4	L	125	2.0	0.757	52.6	LOS D	20.0	142.3	0.97	0.89	24.8
5	Т	105	2.0	0.757	44.4	LOS D	20.0	142.3	0.97	0.87	25.0
6	R	130	2.0	0.757	52.5	LOS D	20.0	142.3	0.97	0.89	24.9
Approa	ch	360	2.0	0.757	50.2	LOS D	20.0	142.3	0.97	0.88	24.9
North: V	NOODVI	LLE									
7	L	52	2.0	0.109	8.0	LOS A	0.2	1.3	0.12	0.62	49.1
8	Т	1602	2.0	0.768	22.9	LOS B	36.3	258.7	0.85	0.78	35.0
9	R	234	2.0	0.775	51.9	LOS D	14.3	102.0	1.00	0.99	24.7
Approac	ch	1888	2.0	0.775	26.1	LOS B	36.3	258.7	0.85	0.80	33.6
West: V	/ILLAWO	OD									
10	L	135	2.0	0.206	36.7	LOS C	5.4	38.6	0.73	0.78	29.9
11	Т	25	2.0	0.061	37.3	LOS C	1.1	7.9	0.80	0.58	28.5
12	R	110	2.0	0.572	59.2	LOS E	6.1	43.4	0.96	0.80	22.8
Approa	ch	270	2.0	0.572	45.9	LOS D	6.1	43.4	0.83	0.77	26.4
All Vehi	cles	3686	2.0	0.775	28.0	LOS B	36.3	258.7	0.81	0.76	32.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians										
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective		
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	m		per ped		
P3	Across E approach	53	15.0	LOS B	0.1	0.1	0.50	0.50		
P7	Across W approach	53	17.6	LOS B	0.1	0.1	0.54	0.54		
All Pede	estrians	106	16.3	LOS B			0.52	0.52		

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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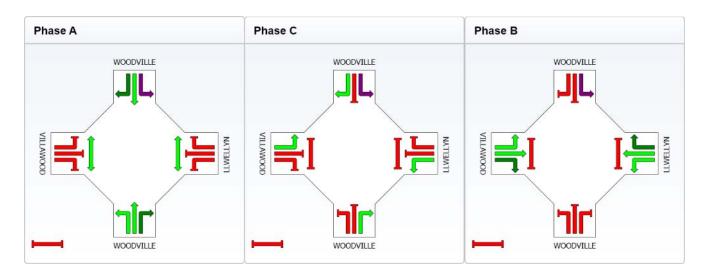
Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program

Sequence: Two-Phase Input Sequence: A, C, B Output Sequence: A, C, B

**Phase Timing Results** 

Phase	Α	С	В
Green Time (sec)	87	6	9
Yellow Time (sec)	4	4	4
All-Red Time (sec)	2	2	2
Phase Time (sec)	93	12	15
Phase Split	78 %	10 %	13 %





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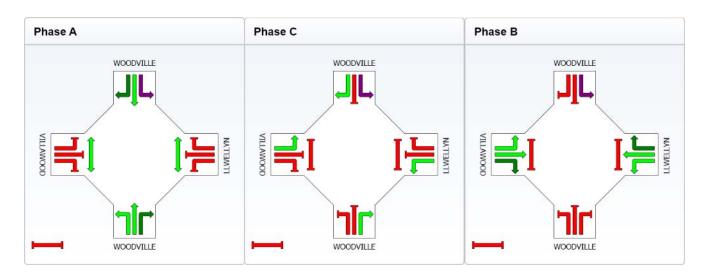
Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program

Sequence: Two-Phase Input Sequence: A, C, B Output Sequence: A, C, B

**Phase Timing Results** 

Phase	Α	С	В
Green Time (sec)	66	7	29
Yellow Time (sec)	4	4	4
All-Red Time (sec)	2	2	2
Phase Time (sec)	72	13	35
Phase Split	60 %	11 %	29 %





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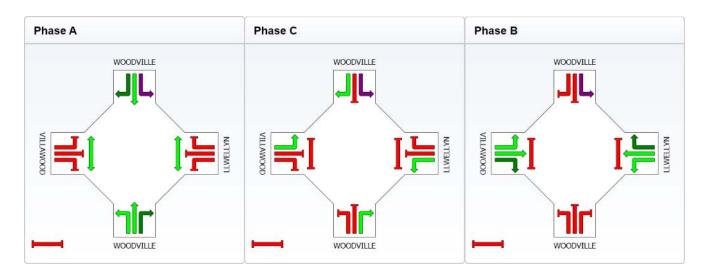
Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program

Sequence: Two-Phase Input Sequence: A, C, B Output Sequence: A, C, B

**Phase Timing Results** 

Phase	Α	С	В
Green Time (sec)	87	6	9
Yellow Time (sec)	4	4	4
All-Red Time (sec)	2	2	2
Phase Time (sec)	93	12	15
Phase Split	78 %	10 %	13 %





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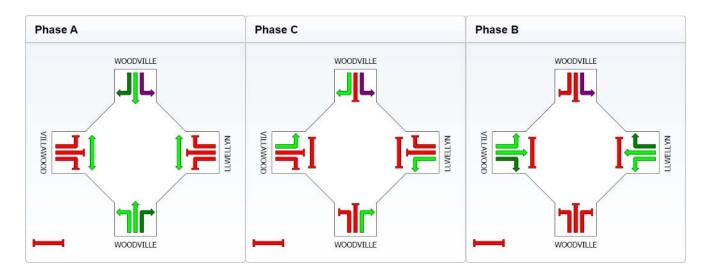
Signals - Fixed Time Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program

Sequence: Two-Phase Input Sequence: A, C, B Output Sequence: A, C, B

**Phase Timing Results** 

Phase	Α	С	В
Green Time (sec)	65	8	29
Yellow Time (sec)	4	4	4
All-Red Time (sec)	2	2	2
Phase Time (sec)	71	14	35
Phase Split	59 %	12 %	29 %





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## **APPENDIX C**

**TURNING PATH ASSESSMENT** 

